

PERVIOUS ELEMENT MANUFACTURER:

MAINTENANCE PROGRAM:

PERVIOUS ELEMENT SLOPE AND DIRECTION OF SLOPE: _

PERVIOUS ELEMENT CROSS SECTION LOCATED IN SHEET:

CONSTRUCTED PERVIOUS SURFACES SHALL NOT BE SEALED

OCCUPANCY GROUP: B

CONSTRUCTION TYPE: V

(805

EMAIL: Pct.2020@GMAIL.COM

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P0014 SHEET NO.

SP-

- PROHIBITED WITHIN 2 FEET OF PROPERTY LINE

- 1-HOUR FIRE RATING ON THE UNDERSIDE WITHIN 3FT OF PROPERTY LINE - 1-HOUR FIRE RATING ON THE UNDERSIDE WITHIN 5FT OF PROPERTY LINE (WITHOUT SPRINKLERS)

OPENINGS:

- PROHIBITED WITHIN 3FT OF PROPERTY LINE - MAXIMUM 25% OF WALL AREA WITHIN 5 FEET OF PROPERTY LINE (WITHOUT SPRINKLERS)

4. PENETRATIONS:

- 1-HOUR FIRE-RATED PENETRATIONS OF WALLS WITHIN 3FT OF PROPERTY LINE (SPRINKLERS)

- 1-HOUR FIRE-RATED PENETRATIONS OF WALLS WITHIN 5FT OF PROPERTY LINE (WITHOUT SPRINKLERS)

. CONCRETE LANDING WITH MIN 36" DEPTH AND A MAXIMUM OF 1-1/2" LOWER THAN TOP OF DOOR THRESHOLD

PLUMBING NOTES

- 1. MIN. ¼" PER FOOT SLOPE FOR WASTE PIPES PER SECTION 708 CPC
- 2. BUILDING DRAIN AND VENT PIPING MATERIALS SHALL COMPLY WITH SECTIONS 701.0 AND 903.0 OF THE CALIFORNIA PLUMBING CODE.
- 3. ALL SANITARY SYSTEM MATERIALS SHALL BE LISTED BY AN APPROVED
- EACH VENT SHALL RISE VERTICALLY TO A POINT NOT LESS THAN SIX(6) INCHES ABOVE THE FLOOD LEVEL RIM OF THE FIXTURE SERVED BEFORE OFFSETTING HORIZONTALLY OR BEFORE BEING CONNECTED TO ANY OTHER
- 5. ALL DRAINAGE WASTE AND VENT PIPE SHALL COMPLY WITH TABLE 703.2 CPC.
- 6. SHOWER AND TUB-SHOWER COMBINATIONS SHALL BE PROVIDED WITH MIXING VALVES PER SECTION 408.3 CPC.
- 7. TOILETS SHALL BE ULTRA-LOW FLUSH TYPE (1.28 G.P.F. MAX.)
- 8. EACH SHOWERHEAD SHALL NOT EXCEED A WATER FLOW OF 1.8 GPM.
- 9. KITCHEN SINK FAUCET SHALL NOT EXCEED A WATER FLOW 1.8 GPM.
- 10. EACH LAVATORY FAUCET SHALL NOT EXCEED A WATER FLOW OF 1.2 GPM.
- 11. ALL SANITARY SYSTEM MATERIALS SHALL BE LISTED BY AN APPROVED LISTING AGENCY.

BATHROOM ACCESSIBILITY:

- REBUILDING AND REPAIRING ROOF TRUSESS DUE TO FIRE DAMAGES
- 2. EXISTING BATHROOM IS LOCATED IN THE MACHINE ROOM AND IT IS NOT ACCESSIBLE TO THE GENERAL PUBLIC AND/OR TENANTS.
- 3. EMPLOYEE ONLY RESTROOM
- 4. BUILDING DRAIN AND VENT PIPING MATERIALS SHALL COMPLY WITH SECTIONS 701.0 AND 903.0 OF THE CALIFORNIA PLUMBING CODE.

DESIGNER: LEEONEL SOLIS EMAIL: LEEONEL28@GMAIL PHONE: 619-414-8506

WINDOW SCHEDULE							
MARK	DIMENSION	TYPE	TEMPERED	NOTES			
A	3'-0" x 4'-0"	SLIDING					

5'-4"

1'-6" —

O (E) LAV.

BATH

(4'-3" x 5'-4") 8'-0" CEILING

EXTERIOR WINDOWS, EXTERIOR GLAZED DOORS, GLAZED OPENINGS WITHIN EXTERIOR DOORS, GLAZED OPENINGS WITHIN EXTERIOR GARAGE DOORS, AND EXTERIOR STRUCTURAL GLASS VENEER SHALL COMPLY WITH ONE OF THE FOLLOWING: (SELECT ONE)

- A. MULTI-PANE GLAZING WITH A MINIMUM OF ONE TEMPERED PANE MEETING THE REQUIREMENTS OF SECTION 2406 SAFETY GLAZING, AND WHERE ANY GLAZING FRAMES MADE OF VINYL MATERIALS SHALL HAVE WELDED CORNERS, METAL REINFORCEMENT IN INTERLOCK AREA, AND BE CERTIFIED TO AAMA/WDMA/CSA 101/I.S.2/A40
- MINIMUM 20-MIN FIRE-RESISTANCE-RATED.

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A-6

C. MEET PERFORMANCE REQUIREMENTS OF SFM STANDARD 12-7A-2

DOOR SCHEDULE TEMPERED DIMENSION TYPE NOTES 1-3/8" SOLID CORE 3'-0" x 6'-8" SWINGING

EXTERIOR DOORS SHALL COMPLY WITH ONE OF THE FOLLOWING: (SELECT ONE)

A. EXTERIOR SURFACE OR CLADDING OF NON-COMBUSTIBLE OR

(A) 4'-11"

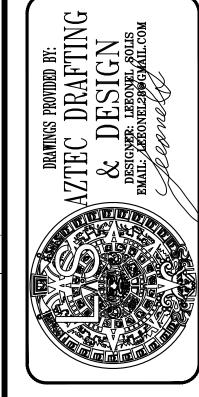
 $18'-6\frac{1}{2}"$

- **IGNITION-RESISTANT MATERIAL** SOLID CORE WOOD COMPLYING WITH THE FOLLOWING:
- STILES AND RAILS MINIMUM 1-3/8 INCHES THICK - RAISED PANELS MINIMUM 1-1/4 INCHES THICK
- A TONGUE MINIMUM 3/8 INCHES THICK

EXCEPTION: EXTERIOR PERIMETER OF RAISED PANEL MAY TAPER TO

- MINIMUM 20-MIN FIRE RATED WHEN TESTED PER NFPA 252
- MEET PERFORMANCE REQUIREMENTS OF SFM STANDARD 12-7A-1

9119 JAMACHA RD, SUITE 11 SPRING VALLEY, CA 91977 CELL: 619-414-8506



J. TULAGAN IMPROVEMENT NATIONAL CITY CA 91950 369-101-09-00

REVISION 02/09/202 08/19/2022 10/31/2022

> PROJECT NO. P0014 SHEET NO.

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EXISTING FLOOR PLAN

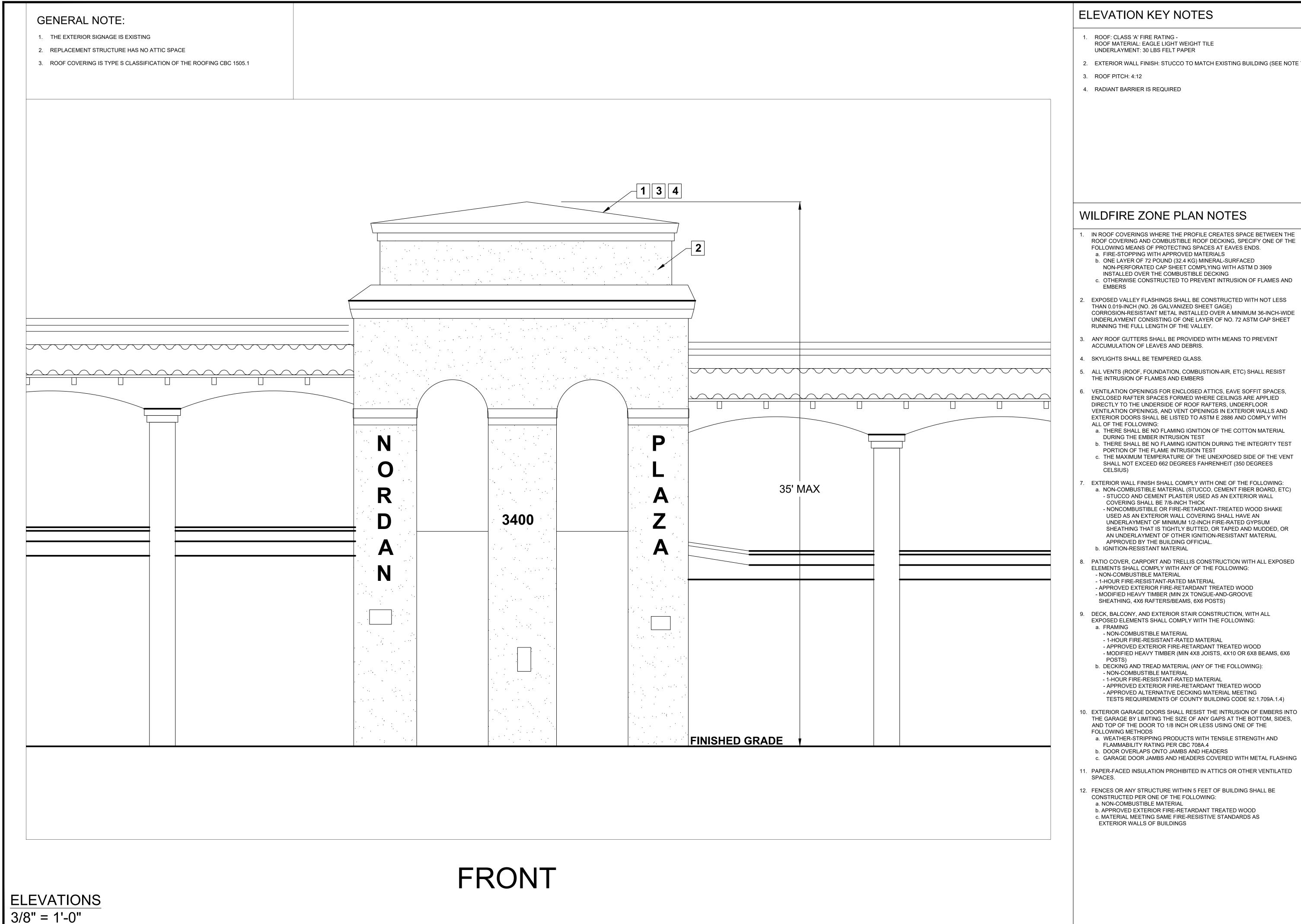
WALL LEGEND

2x4 WALL

2x4 PONY WALL

2x6 WALL

1/2" = 1'-0"



ELEVATION KEY NOTES

- ROOF: CLASS 'A' FIRE RATING -ROOF MATERIAL: EAGLE LIGHT WEIGHT TILE UNDERLAYMENT: 30 LBS FELT PAPER
- 2. EXTERIOR WALL FINISH: STUCCO TO MATCH EXISTING BUILDING (SEE NOTE 7 BELOW)

NON-PERFORATED CAP SHEET COMPLYING WITH ASTM D 3909

a. THERE SHALL BE NO FLAMING IGNITION OF THE COTTON MATERIAL

b. THERE SHALL BE NO FLAMING IGNITION DURING THE INTEGRITY TEST

c. THE MAXIMUM TEMPERATURE OF THE UNEXPOSED SIDE OF THE VENT SHALL NOT EXCEED 662 DEGREES FAHRENHEIT (350 DEGREES

a. NON-COMBUSTIBLE MATERIAL (STUCCO, CEMENT FIBER BOARD, ETC)

- NONCOMBUSTIBLE OR FIRE-RETARDANT-TREATED WOOD SHAKE USED AS AN EXTERIOR WALL COVERING SHALL HAVE AN

- STUCCO AND CEMENT PLASTER USED AS AN EXTERIOR WALL

UNDERLAYMENT OF MINIMUM 1/2-INCH FIRE-RATED GYPSUM SHEATHING THAT IS TIGHTLY BUTTED, OR TAPED AND MUDDED, OR AN UNDERLAYMENT OF OTHER IGNITION-RESISTANT MATERIAL

DURING THE EMBER INTRUSION TEST

CELSIUS)

a. FRAMING

PORTION OF THE FLAME INTRUSION TEST

COVERING SHALL BE 7/8-INCH THICK

APPROVED BY THE BUILDING OFFICIAL.

- 1-HOUR FIRE-RESISTANT-RATED MATERIAL

SHEATHING, 4X6 RAFTERS/BEAMS, 6X6 POSTS)

- 1-HOUR FIRE-RESISTANT-RATED MATERIAL

- 1-HOUR FIRE-RESISTANT-RATED MATERIAL

FLAMMABILITY RATING PER CBC 708A.4 b. DOOR OVERLAPS ONTO JAMBS AND HEADERS

a. NON-COMBUSTIBLE MATERIAL

EXTERIOR WALLS OF BUILDINGS

- APPROVED EXTERIOR FIRE-RETARDANT TREATED WOOD - MODIFIED HEAVY TIMBER (MIN 2X TONGUE-AND-GROOVE

- APPROVED EXTERIOR FIRE-RETARDANT TREATED WOOD

- APPROVED EXTERIOR FIRE-RETARDANT TREATED WOOD - APPROVED ALTERNATIVE DECKING MATERIAL MEETING

a. WEATHER-STRIPPING PRODUCTS WITH TENSILE STRENGTH AND

b. APPROVED EXTERIOR FIRE-RETARDANT TREATED WOOD

c. MATERIAL MEETING SAME FIRE-RESISTIVE STANDARDS AS

- MODIFIED HEAVY TIMBER (MIN 4X8 JOISTS, 4X10 OR 6X8 BEAMS, 6X6

TESTS REQUIREMENTS OF COUNTY BUILDING CODE 92.1.709A.1.4)

c. GARAGE DOOR JAMBS AND HEADERS COVERED WITH METAL FLASHING

b. IGNITION-RESISTANT MATERIAL

- NON-COMBUSTIBLE MATERIAL

- NON-COMBUSTIBLE MATERIAL

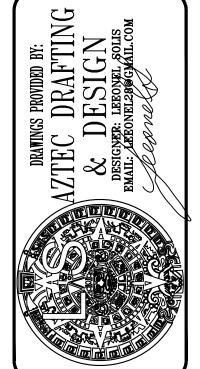
- NON-COMBUSTIBLE MATERIAL

c. OTHERWISE CONSTRUCTED TO PREVENT INTRUSION OF FLAMES AND

INSTALLED OVER THE COMBUSTIBLE DECKING

- ROOF PITCH: 4:12
- 4. RADIANT BARRIER IS REQUIRED





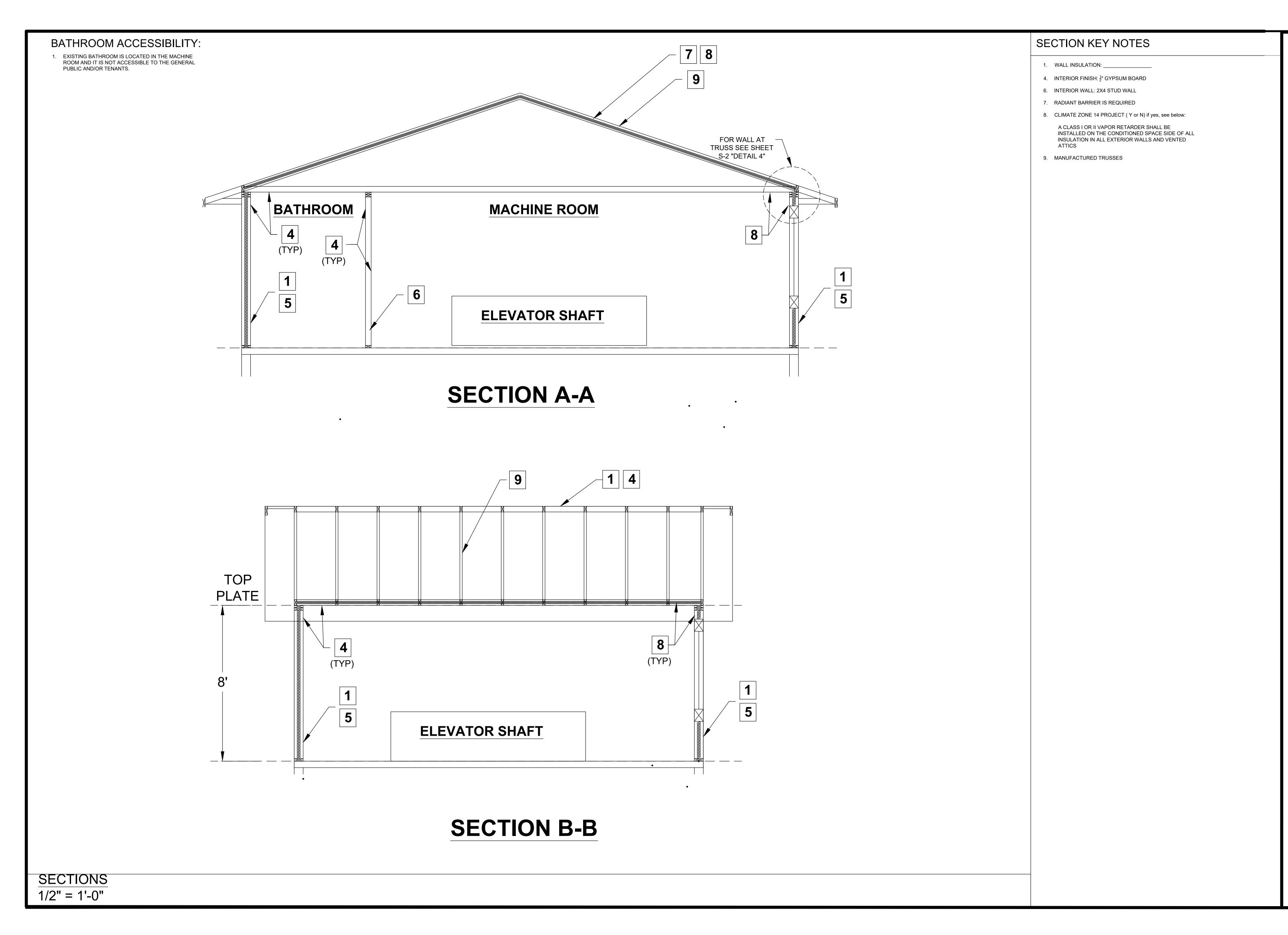
J. TULAGAN IMPROVEMENT , NATIONAL CITY CA 91950 669-101-09-00 TILITY: SDG&E NATIONAL CITY

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PROJECT NO. P0014 SHEET NO.

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AZTEC DRAFTING
& DESIGN
9119 JAMACHA RD, SUITE 115
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DANILO J. TULAGAN TENANT IMPROVEMENT 3400 e 8th St, National city ca 91950 APN: 669-101-09-00 UTILITY: SDG&E

SECTION VIEW

REVISION

0 - 02/09/2022

1 - 08/19/2022

1 - 10/31/2022

PROJECT NO.

P0014

SHEET NO.

A-6

C. TEMPORARY WORK AND SITE SAFETY:

1. THESE DRAWINGS SHOW THE REQUIREMENTS FOR PERMANENT COMPLETED STRUCTURE ONLY. TEMPORARY WORKS REQUIRED TO COMPLETE THE CONSTRUCTION SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR DESIGN OR FIELD REVIEW OF TEMPORARY AND ANCILLARY WORK.

2. THE CONTRACTOR IS RESPONSIBLE FOR SAFETY IN AND AROUND THE JOBSITE. PROPER AND SAFE METHODS OF CONSTRUCTION SHALL BE USED AT ALL TIMES INCLUDING GUYING AND BRACING OF INCOMPLETE STRUCTURES. FORMWORK, SHORING, RESHORING, FALSEWORK, PLATFORMS, SCAFFOLDING, BARRIERS, WALKWAYS, ETC. AND CONTROL THE INTENSITY, DURATION AND LOCATION OF CONSTRUCTION LOADS UPON CONSTRUCTION.

1. THE STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE 2016 CALIFORNIA BUILDING CODE (C.B.C.)

2. LIVE LOADS (REDUCED IN ACCORDANCE WITH THE 2016 C.B.C.)

SLOPED ROOF	20 p
FLOOR	40 ps

A. BASIS OF DESIGN

3. LATERAL LOADS & CRITERIA	
BUILDING SITE CLASS	D
LATITUDE LONGITUDE	32.6834 -117.1062
SEISMIC DESIGN CATEGORY	D
S _S	1.054
S ₁	0.401
S _{DS}	0.7578
S _{D1}	0.4275
R	6.50
Ω_{O}	3.0
^-	4.0

STRUCTURAL SYSTEM: BEARING WALLS SYSTEM LIGHT FRAMED WALLS SHEATHED WITH WOOD STRUCTURAL PANELS RATED FOR SHEAR

0.02

0.75

RESISTANCE OR STEEL SHEETS.	
SEISMIC SOURCE TYPE	В
SOIL PROFILE	Sd
IMPORTANCE FACTOR	1.0
WIND EXPOSURE CATEGORY	В
ROOF ANGLE	15 [°]
BASIC WIND SPEED	110 mph

B. GENERAL NOTES:

1. THE CONTRACTOR SHALL VERIFY DIMENSIONS, ELEVATIONS, AND SITE CONDITIONS BEFORE STARTING ANY WORK AND

2. COORDINATE ELEVATIONS. SLOPES AND DRAINAGE NOTIFY THE ARCHITECT/ENGINEER IMMEDIATELY OF ANY DISCREPANCIES. REQUIREMENTS WITH THE ARCHITECTURAL DRAWINGS. 3. SPECIFIC NOTES AND DETAILS ON DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS.

4. WHERE NO DETAILS ARE SHOWN OR NOTED IN ANY PART OF THE WORK THE DETAILS USED SHALL BE THE SAME AS FOR OTHER SIMILAR WORK.

5. WHEN A DETAIL IS IDENTIFIED AS TYPICAL, THE CONTRACTOR IS TO APPLY THIS DETAIL IN ESTIMATING AND CONSTRUCTION TO EVERY LIKE CONDITION WHETHER OR NOT THE REFERENCE IS REPEATED IN EVERY INSTANCE

6. THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ARCHITECT/ENGINEER OF ANY SITE CONDITIONS NOT REFLECTED ON THE WORKING DRAWINGS OR DIFFERENT FROM THE MAXIMUM OR MINIMUM DIMENSIONS INDICATED. INCLUDING CONFLICT IN GRADES, ADVERSE SOIL CONDITIONS, GROUND WATER PRESENT, DEEPENED FOOTINGS, UNCOVERED AND UNEXPECTED UTILITY LINES, ETC.

7. ALL DRAWINGS ARE CONSIDERED TO BE A PART OF THE CONTRACT DOCUMENTS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE REVIEW AND COORDINATION OF ALL DRAWINGS AND SPECIFICATIONS PRIOR TO THE START OF CONSTRUCTION. ANY DISCREPANCIES THAT OCCUR SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT PRIOR TO START OF CONSTRUCTION SO THAT A CLARIFICATION CAN BE ISSUED. ANY WORK PERFORMED IN CONFLICT WITH THE CONTRACT DOCUMENTS OR ANY CODE REQUIREMENTS SHALL BE CORRECTED BY THE CONTRACTOR AT THEIR OWN EXPENSE.

8. MATERIALS AND WORKMANSHIP SHALL CONFORM TO REQUIREMENTS OF THE CURRENT CALIFORNIA BUILDING CODE AS AMENDED BY THE GOVERNING AUTHORITY AND APPLICABLE REGULATIONS OF THE GOVERNING JURISDICTION, INCLUDING THE STATE OF CALIFORNIA DIVISION OF INDUSTRIAL SAFETY.

9. DRAWINGS SHALL NOT BE SCALED. COORDINATE DIMENSIONS WITH ARCHITECTURAL DRAWINGS.

10. ASTM SPECIFICATIONS ON THE DRAWINGS SHALL BE OF THE LATEST REVISION.

11. CONSTRUCTION MATERIALS SHALL BE SPREAD OUT IF PLACED ON FRAMED FLOOR. LOADS SHALL NOT EXCEED THE DESIGN LIVE LOAD PER SQUARE FOOT. PROVIDE ADEQUATE SHORING AND/OR BRACING WHERE STRUCTURE HAS NOT ATTAINED DESIGN STRENGTH.

D. FOUNDATION

1. FOUNDATION DESIGN IS BASED ON THE 2016 CBC.

2. ALLOWABLE BEARING SOIL PRESSURE: 1,500 psf 3. THE MAXIMUM ALLOWABLE SOIL BEARING PRESSURE SHALL BE 1,500 psf. ALLOWABLE BEARING MAY BE INCREASED BY 1/3 FOR WIND AND SEISMIC LOAD CASES.

4. BOTTOM OF FOOTING SHALL BE EMBEDDED AT LEAST 12 INCHES BELOW LOWEST ADJACENT FINISHED (PAD) GRADE.

5. FOOTING DEPTHS SHOWN ARE FOR BIDDING PURPOSES ONLY AND ARE ASSUMED TO BE IN SUITABLE BEARING MATERIALS. FOOTING DEPTHS MAY REQUIRE DEEPENING PER DIRECTION OF THE ENGINEER.

6. ABANDONED FOOTINGS, UTILITIES, ETC. THAT INTERFERE WITH NEW CONSTRUCTION SHALL BE REMOVED.

7. THE FOOTING EXCAVATIONS SHALL BE KEPT FREE FROM LOOSE MATERIAL AND STANDING WATER. CONTRACTOR SHALL PROVIDE FOR DE-WATERING OF EXCAVATIONS FROM SURFACE OR SEEPAGE WATER. 8. FOOTING AND UTILITY TRENCH BACKFILL SHALL BE

MECHANICALLY COMPACTED IN LAYERS. FLOODING WILL NOT BE PERMITTED. 9. SUBMIT COMPACTION TEST REPORTS FOR ALL FILL BY A

QUALIFIED TESTING LAB TO ENGINEER AND BUILDING DEPARTMENT PRIOR TO REQUESTING FOUNDATION INSPECTION.

10. CONTRACTOR SHALL PROVIDE FOR DESIGN AND INSTALLATION OF ALL CRIBBING, SHEATHING, UNDERPINNING, AND SHORING REQUIRED TO SAFELY RETAIN ALL GRADES AND STRUCTURES.

11. FOOTING ELEVATIONS SHOULD BE LOCATED SUCH THAT THE BASES OF THE FOUNDATIONS ARE A MINIMAL HORIZONTAL DISTANCE OF SEVEN FEET FROM THE FACE OF SLOPE.

12. SLAB ON GRADE RESTRAINING THE BOTTOM OF RETAINING WALLS SHALL BE IN PLACE PRIOR TO BACKFILLING OF WALLS.

13. WALLS RETAINING EARTH SHALL BE DRAINED TO DAYLIGHT OR DRAINAGE STRUCTURE AND BACKFILLED PER SOIL ENGINEER'S RECOMMENDATION.

14. FOUNDATIONS SUPPORTING WOOD SHALL EXTEND 8" MINIMUM ABOVE ADJACENT FINISH GRADE. PROVIDE 18" CLEARANCE UNDER WOOD JOISTS AND 18" CLEARANCE UNDER WOOD GIRDERS.

E. REINFORCING STEEL

1. DETAILING, FABRICATION AND ERECTION OF REINFORCING BARS MUST FOLLOW THE A.C.I. MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES, A.C.I. 315-LATEST ED. U.O.N.

2. REINFORCING BARS SHALL CONFORM TO THE 2016 CBC AND THE STANDARD SPECIFICATION FOR DEFORMED BILLET-STEEL BARS FOR CONCRETE REINFORCEMENT. ASTM DESIGNATION A-615, GRADE 60, U.O.N.

3. LAPS AT BAR SPLICES SHALL BE: 42 BAR DIA. (18" MIN.) FOR CONCRETE U.O.N. 48 BAR DIA. (24" MIN.)

4. REINFORCING BARS SHALL BE PROVIDED WITH THE FOLLOWING CONCRETE COVER:

CONC. CAST AGAINST EARTH FORMED CONC. EXPOSED TO EARTH/WEATHER #5 OR LARGER 1-1/2" #6 OR LARGER SLABS (#11 AND SMALLER)

5. VERTICAL BARS SHALL BE ACCURATELY POSITIONED AT THE CENTER OF THE WALL, U.O.N. ON DETAILS, AND SHALL BE TIED IN PLACE AT THE TOP AND BOTTOM.

6. PROVIDE #3 SPACER TIES AT 30" (75 mm) ON CENTER IN ALL BEAMS AND FOOTINGS TO SECURE REINFORCING BARS IN PLACE, U.O.N.

F. REINFORCED CONCRETE (GENERAL)

1. CONCRETE CONSTRUCTION SHALL CONFORM WITH CHAPTER 19 OF THE CODE AND WITH THE PROVISIONS OF ACI 318, LATEST EDITION. 2. CONCRETE MIXES SHALL BE DESIGNED BY A QUALIFIED TESTING LABORATORY AND APPROVED BY THE STRUCTURAL ENGINEER.

3. CEMENT SHALL CONFORM TO ASTM C-150 TYPE I OR II, ALKALI (2016 CBC).

5. READY MIX CONCRETE SHALL BE MIXED AND DELIVERED IN

ACCORDANCE WITH ASTM C-94.

6. MINIMUM CONCRETE COMPRESSIVE STRENGTHS AT 28 DAYS, MAXIMUM SLUMPS, AND MAXIMUM WATER/CEMENT RATIOS SHALL BE AS FOLLOWS:

*CONCRETE HAS BEEN DESIGNED FOR 2,500 psi. NO INSPECTION IS REQUIRED.

7. WATER MAY BE ADDED ON SITE TO OBTAIN SPECIFIED SLUMPS ONLY IF IT IS ADDED WITHIN ONE HOUR OF BATCHING AND SPECIFIED ON THE BATCH REPORT. CONCRETE SHALL NOT BE PLACED BEYOND 1-1/2 HOURS FOLLOWING BATCHING.

8. NO PIPES OR DUCTS SHALL BE PLACED IN CONCRETE SLABS UNLESS SPECIFICALLY DETAILED. PROVIDE SLEEVES FOR PLUMBING AND ELECTRICAL OPENINGS IN CONCRETE BEFORE PLACING. DO NOT CUT ANY REINFORCING WHICH MAY CONFLICT.

9. CONCRETE SHALL BE MAINTAINED IN A MOIST CONDITION FOR A MINIMUM OF SEVEN DAYS AFTER ITS PLACEMENT. APPROVED CURING COMPOUNDS MAY BE USED IN LIEU OF MOIST CURING.

10. CONCRETE SLAB-ON-GRADE THICKNESS SHOWN IS MINIMUM REQUIRED THICKNESS. FLOORS SHALL BE MONITORED BY TRANSIT LEVEL OR LASER DURING PLACEMENT TO MAINTAIN

11. FLYASH SHALL BE LIMITED TO NO MORE THAN 15 PERCENT OF THE TOTAL WEIGHT OF CEMENTIOUS CONCRETE, U.O.N. 12. CONCRETE EXPOSED TO WEATHER SHALL BE AIR ENTRAINED.

STRUCTURAL NOTES

13. PROVIDE 1-#5 x 4'-0" LONG DIAGONAL BAR AT CORNERS OF WALL, FLOOR, AND ROOF OPENINGS AND INSIDE CORNERS OF CONCRETE FLOORS.

> MAKING 45 DEGREES WITH ANY PARALLEL OR PENPENDICULAR LINE OF OPENING.

14. ALL CONSTRUCTION JOINTS IN STRUCTURAL MEMBERS TO BE REVIEWED FOR LOCATION AND DETAIL PRIOR TO CONSTRUCTION. FLEXURAL REINFORCEMENT TO CONTINUE UNINTERRUPTED THROUGH ALL CONSTRUCTION JOINTS. KEYWAYS TO BE PROVIDED PERPENDICULAR TO THE DIRECTION OF LOAD IN ALL JOINTS.

15. WHEN CONCRETE IS PLACED AGAINST EXISTING CONCRETE SURFACES, EXISTING CONCRETE SURFACES SHALL BE THOROUGHLY CLEANED AND THEN SANDBLASTED TO CREATE AN AMPLITUDE OF 1/4" MINIMUM, APPLY A CONCRETE BONDING ADJACENT TO IMPROVE BONDING QUALITY.

G. ROUGH CARPENTRY:

1. ROOF SHEATHING SHALL BE APA RATED AND SHALL CONFORM TO PRODUCT STANDARD PS 1, INTERIOR TYPE WITH EXTERIOR GLUE, IDENTIFICATION INDEX (24/0) UNLESS OTHERWISE NOTED. EQUIVALENT THICKNESS O.S.B. BOARD MAY BE USED IN LIEU OF PLYWOOD ROOF SHEATHING.

2. FLOOR SHEATHING, THICKNESS, GRADE, AND NAILING PER STRUCTURAL PLANS. PLYWOOD SHEATHING SHALL CONFORM TO PRODUCT STANDARD PS 1-80, TONGUE AND GROOVE, INTERIOR TYPE WITH EXTERIOR GLUE, IDENTIFICATION INDEX (32/16). O.S.B. BOARD SHALL NOT BE USED IN LIEU OF PLYWOOD FLOOR SHEATHING.

3. SHEATHING SHALL BE LAID PERPENDICULAR TO FRAMING FOR FLOORS AND ROOFS WITH 4' JOINTS STAGGERED AND CENTERED ON JOISTS. ALL OTHER JOINTS AT FLOORS SHALL BE BLOCKED.

4. PLYWOOD FLOOR SHEATHING SHALL BE GLUED TO ALL FRAMING MEMBERS WITH AN A.P.A. APPROVED ADHESIVE.

5. UNLESS OTHERWISE NOTED, ALL FRAMING LUMBER SHALL BE DOUGLAS FIR LARCH, GRADE-MARKED BY THE W.C.L.I.B. OR W.W.P.A. AS FOLLOWS:

2X JOISTS & RAFTERS NO. 1 4X & LARGER BEAMS NO. 1 ALL POSTS NO. 1 NO. 2 STUDS PLATES NO. 2 LEDGERS NO. 1 BLOCKING 1 NO. 3 PLYWOOD A.P.A. RATED O.S.B. BOARD A.P.A. RATED

6. SILL PLATES SHALL BE TREATED DOUGLAS FIR OR FOUNDATION REDWOOD. EXTERIOR WALL SILL PLATES SHALL BE SECURED TO CONCRETE WITH 5/8" X 10" LONG ANCHOR BOLTS WITH 7" MINIMUM EMBEDMENT INTO CONCRETE AT A MAXIMUM SPACING OF 48" O.C. AND 12" FROM EACH END. PLATE WASHERS A MINIMUM OF 3 INCH BY 3 INCH BY 1/4 OF AN INCH THICK SHALL BE USED ON EACH BOLT. (FOR SPECIAL CONDITIONS, SEE SHEAR WALL SCHEDULE FOR SHEAR WALL ANCHORAGE).

7. DO NOT BORE OR NOTCH JOISTS, RAFTERS, OR BEAMS, EXCEPT WHERE SHOWN IN DETAILS. OBTAIN ENGINEER'S APPROVAL FOR ANY HOLES OR NOTCHES NOT DETAILED.

8. PROVIDE DOUBLE FLOOR JOISTS UNDER PARALLEL PARTITIONS, U.O.N.

9. PROVIDE 1/2 INCH MINIMUM CLEARANCE BETWEEN TOP PLATES OF INTERIOR NON-BEARING PARTITIONS AND THE BOTTOM CHORD OF TRUSSES.

10. NAILS SHALL BE COMMON WIRE. NAILING SHALL COMPLY WITH TABLE 23-II-B-1 OF THE 2016 C.B.C. NAILS EXPOSED TO WEATHER SHALL BE HOT-DIP GALVANIZED, U.O.N. 11. PROVIDE SOLID BLOCKING AT ENDS AND AT SUPPORTS OF FLOOR JOISTS AND ROOF RAFTERS UNDER PARTITIONS AND AT RIDGE LINE.

12. TOP PLATES OF ALL BEARING WOOD STUD WALLS SHALL BE TWO PIECES, SAME SIZE AS STUDS AND LAPPED 4'-0" MINIMUM WITH NOT LESS THAN 10-16d NAILS AT EACH SIDE OF TOP PLATE BREAK POINT SPACED AT 4" O.C. MAXIMUM UNLESS OTHERWISE NOTED.

13. INTERIOR AND EXTERIOR WOOD POSTS ATTACHED DIRECTLY TO CONCRETE SHALL BE SECURED WITH SIMPSON PB OR EPB POST BASES, AS APPLICABLE, UNLESS OTHERWISE NOTED. 14. STUDS SHALL HAVE FULL BEARING ON PLATE, ALL JOISTS, HEADERS, BEAMS, AND RAFTERS SHALL HAVE A MINIMUM SOLID LEVEL BEARING OF 1.5 INCHES AT EACH END.

15. NOT LESS THAN THREE (3) STUDS SHALL BE INSTALLED AT EVERY CORNER OF AN EXTERIOR OR INTERIOR BEARING WALL. 16. BEAMS, JOISTS, RAFTERS, ETC. SHALL BE INSTALLED

WITH THE CROWN SIDE UP. 17. BOLT HOLES IN WOOD SHALL BE DRILLED 1/32" TO 1/16" IN DIAMETER LARGER THAN THE NOMINAL BOLT SIZE. RETIGHTEN ALL NUTS PRIOR TO CLOSING IN.

18. LAG BOLTS SHALL BE PRE-DRILLED TO A DIAMETER OF 60 PERCENT OF THE SHANK DIAMETER. THE BOLT SHALL BE

TURNED BY A WRENCH AND NOT HAMMERED. 19. BOLTS SHALL HAVE A 7 DIA. MIN. END DISTANCE AND A 4 DIA. EDGE DISTANCE, U.O.N.

20. STANDARD CUT WASHERS SHALL BE USED UNDER ALL BOLT HEADS AND NUTS AGAINST WOOD. USE HEAVY PLATE OR MALLEABLE IRON WASHERS FOR ALL BOLTS DESIGNED TO ACT IN TENSION, SUCH AS LEDGERS AND HOLD DOWN ANCHORS. 21. PROVIDE FIRE BLOCKING OR JOINT BLOCKING BETWEEN STUDS AT NOT LESS THAN 8'-0" VERTICAL INTERVALS AND AT ALL PLYWOOD EDGES.

22. FRAMING ANCHORS, POST CAPS, COLUMN BASES, HANGERS, ETC. SHALL BE MANUFACTURED BY SIMPSON, OR APPROVED EQUAL.

23. PROVIDE 2X MINIMUM BACKING FOR ALL WALL HUNG CABINETS, HANDRAILS, SHELVING, LIGHT FIXTURES, ACCESSORIES, ETC. 24. PRESSURE TREATED DOUGLAS FIR SHALL BE NO. 2 MINIMUM

AND BEAR "A.W.P.B." QUALITY MARK AND THE W.C.L.N.G.

GRADE STAMP. CERTIFICATES ARE NOT ACCEPTABLE. 25. CUTS AND HOLES IN PRESSURE TREATED LUMBER SHALL BE TREATED PER A.W.P.A. M-84.

H. PREMANUFACTURED ROOF TRUSSES

1. TRUSS DRAWINGS, CALCULATIONS AND THE LATEST ICC-ESR APPROVED TEST DATA FOR TRUSS METAL PLATE CONNECTORS SHALL BE SUBMITTED TO THE ARCHITECT AND/OR ENGINEER FOR REVIEW PRIOR TO FABRICATION. CALCULATIONS FOR GIRDER TRUSSES SHALL INCLUDE POINT LOADS FROM

CARRIED TRUSS REACTIONS. 2. CALCULATIONS AND SHOP DRAWINGS SHALL BE SIGNED BY A CALIFORNIA REGISTERED CIVIL OR STRUCTURAL ENGINEER. IT SHALL BE THE RESPONSIBILITY OF THE MANUFACTURER TO OBTAIN APPROVALS OF FINAL CALCULATIONS AND SHOP DRAWINGS PRIOR TO FABRICATION.

3. TRUSSES SHALL BE DESIGNED IN ACCORDANCE WITH THE LATEST LOCAL APPROVED BUILDING CODES AND ORDINANCES FOR ALL LOADS IMPOSED, INCLUDING LATERAL LOADS. FABRICATOR SHALL REVIEW ALL DRAWINGS AND MEET PROFILES AS INDICATED.

4. THE MANUFACTURER SHALL BE RESPONSIBLE FOR THE DESIGN OF MEMBERS USED AS DRAG OR CHORD MEMBERS AND SHALL INSURE THAT SUCH MEMBERS ARE PLACED AS REQUIRED ON THE FRAMING PLANS. THE AMOUNT OF LOAD TO BE LATERALLY TRANSMITTED BY THE MEMBER SHALL BE A MINIMUM OF 2000 POUNDS U.O.N. ON THE FRAMING PLANS. 5. ROOF TRUSS DESIGN LOADS

DEAD LOAD LIVE LOAD TOP CHORD 17 PSF 16 PSF (REDUCIBLE) BOTTOM CHORD 5 PSF 10 PSF (NON-CONCURRENT

*DESIGN ROOF TRUSSES TO SUPPORT A 500LB. CONCENTRATED LOAD AT ANY TOP CHORD PANEL

6. MAXIMUM FLOOR AND ROOF DEFLECTIONS: MAXIMUM DEFLECTIONS LOCATION LIVE LOAD TOTAL LOAD

ROOF L/360 L/240

MEMBERS, ARE NOT PERMISSIBLE. 8. PROVIDE ADEQUATE CAMBER FOR DESIGNATED DESIGN LOADS.

7. INCREASES IN ALLOWABLE STRESSES FOR REPETITIVE

9. TRUSS DESIGNER SHALL OVERSIZE PLATES FOR CHORD MEMBERS TO ACCOUNT FOR WOOD DEFECTS LIKE KNOTS, KNOT HOLES AND GREATLY DISTORTED GRAINS. MAXIMUM ALLOWABLE DEFECT SIZE PER MEMBER SHALL BE 2 SQUARE INCHES. NO DEFECTS ALLOWED UNDER PLATES FOR WEB MEMBERS.

10. TRUSS MANUFACTURER TO VERIFY ALL DIMENSIONS SHOWN ON STRUCTURAL DRAWINGS WITH ARCHITECTURAL DRAWINGS AND IN FIELD WITH WALL LAYOUT PRIOR TO FABRICATION. PROVIDE SHOP DRAWINGS WHICH SHALL INCLUDE PLAN DRAWING SHOWING TRUSS LOCATIONS AND TRUSS PROFILES. WITH DIMESIONS REVIEWED AND APPROVED BY GENERAL CONTRACTOR, PRIOR TO FABRICATION.

11. GABLE END TRUSSES SHALL HAVE 2X VERTICALS AT 16" O.C.TYPICAL UNLESS OTHERWISE NOTED.

12. SHOP DRAWINGS SHALL BE SUBMITTED TO THE ARCHITECT/ENGINEER FOR REVIEW PRIOR TO FABRICATION AND WILL INCLUDE THE FOLLOWING MINIMUM INFORMATION: a. PROJECT NAME AND LOCATION b. DESIGN LOADS, CONFIGURATIONS, (2 OR 3 POINT BEARING) AND SHEAR TRANSFER. c. MEMBER STRESSES, DEFLECTIONS, TYPE OF JOINT PLATES AND ALLOWABLE DESIGN VALUES.

TRUSS JOINTS SHALL BE DESIGNED FOR 125% OF THE DESIGN STRESSES. d. TYPE, SIZE, AND LOCATION OF HANGERS TO BE USED FOR THE PROJECT. HANGERS SHALL BE DESIGNED TO SUPPORT THE FULL VERTICAL LOAD AND A LATERAL LOAD EQUAL TO 20% OF THE VERTICAL REACTION. ALL CONNECTORS SHALL BE ICBO APPROVED AND OF ADEQUATE STRENGTH TO RESIST STRESSES DUE TO THE LOADING INVOLVED.

13. ALL HARDWARE REQUIRED FOR CONNECTING TRUSSES (JACK TO HIP, HIP TO GIRDER OR GIRDER TO GIRDER, ETC.) SHALL BE DESIGNED, DETAILED AND PROVIDED BY TRUSS FABRICATOR.

14. THE TRUSS MANUFACTURER SHALL BE RESPONSIBLE FOR ALL TRUSS TO TRUSS CONNECTIONS. EACH TRUSS SHALL BE LEGIBLY MARKED WITH THE FOLLOWING INFORMATION WITHIN TWO FEET OF THE CENTER OF THE SPAN ON THE FACE OF THE BOTTOM OF THE CHORD:

1. MANUFACTURER'S NAME 2. DESIGN LOADS 3. TRUSS SPACING

15. MULTIPLE CHORDS SHALL BE FACTORY LAMINATED.

16. CROSS BRIDGING AND/OR BRACING SHALL BE PROVIDED FOR, AND DETAILED BY, THE MANUFACTURER AS REQUIRED TO ADEQUATELY BRACE TRUSSES.

17. WHERE TRUSSES BLOCKING IS CALLED OUT, THE BLOCKING PIECE SHALL BE THE SAME DEPTH AS THE ADJOINING MEMBERS AND CAPABLE OF RESISTING A LATERAL LOAD EQUAL TO 500 POUNDS IN ITS PLANE, OR BE SHEATHED SOLID WITH 1/2" CDX PLYWOOD AND NAILED WITH 10d COMMON NAILS AT 6" (EN) U.O.N. ON THE FRAMING PLANS.

18. GENERAL CONTRATOR TO PROVIDE TEMPORARY ERECTION BRACING AND WEB BRACING AS REQUIRED BY TRUSS MANUFACTURER'S DESIGN.

I. MACHINE APPLIED NAILING:

1. THE USE OF MACHINE NAILING IS SUBJECT TO A SATISFACTORY JOB SITE DEMONSTRATION AND THE APPROVAL OF THE PROJECT ENGINEER. THE APPROVAL IS SUBJECT TO CONTINUED SATISFACTORY PERFORMANCE.

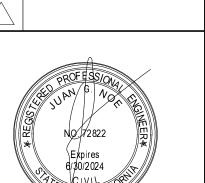
2. NAIL HEADS SHALL NOT PENETRATE THE OUTER PLY MORE THAN WOULD BE NORMAL FOR A HAND HAMMER.

3. EDGE DISTANCES SHALL BE MAINTAINED. SHINERS SHALL BE REPLACED. IF NAIL HEADS PENETRATE THE OUTER PLY MORE THAN WOULD BE NORMAL FOR A HAND HAMMER, OR IF MINIMUM ALLOWABLE EDGE DISTANCES ARE NOT MAINTAINED THE PERFORMANCE WILL BE DEEMED UNSATISFACTORY.

4. MACHINE NAILING WILL NOT BE APPROVED FOR PLYWOOD 5/16" OR LESS IN THICKNESS.

8/8/2022

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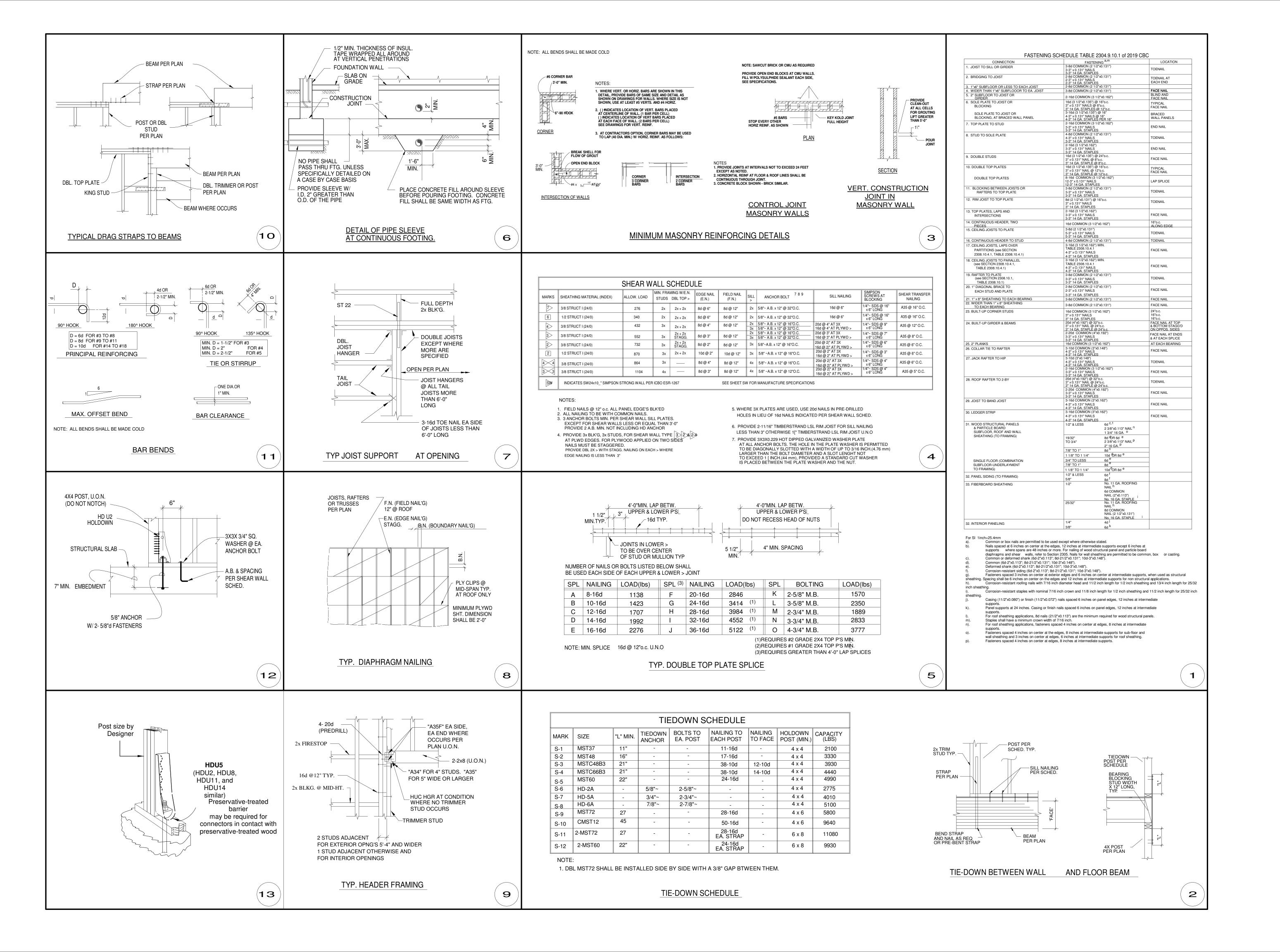


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NORDAN PLAZA

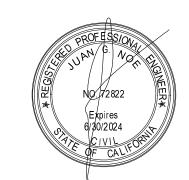
Struct. Notes

OF



TUDIO, 8/8/2022

REVISIONS

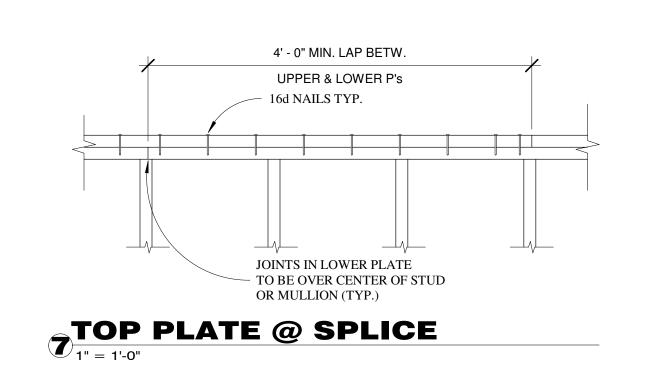


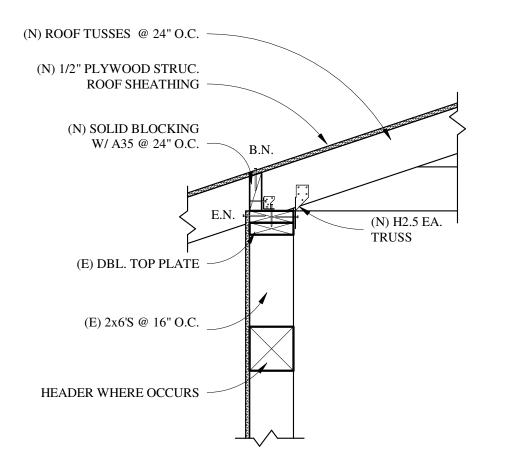
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NORDAN PLAZA

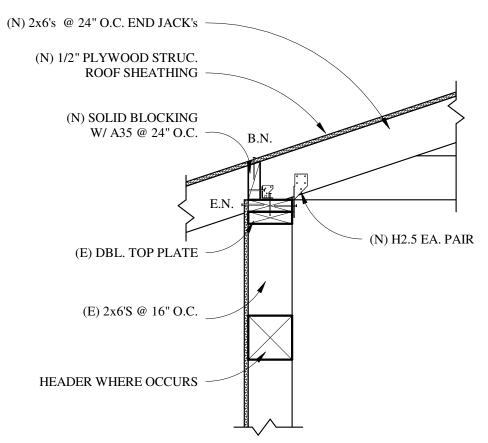
Strut. Details

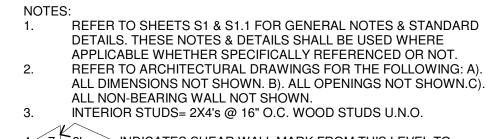
OF SHEET





WALL @ TRUSS1" = 1'-0"





INDICATES SHEAR WALL MARK FROM THIS LEVEL TO
LEVEL ABOVE PER SHEAR WALL SCHEDULE ON S1.1
PROVIDE NON-SHEAR PLYWOOD ADJACENT TO SHEAR
PANELS IN ORDER TO PROVIDE A FLUSH FINISH.

INDICATES SHEAR WALL PANEL APPROX. MIN. LENGTH
IF NOT SHOWN, THEN PROVIDE PLYWOOD ON ENTIRE
FACE.
INDICATES SHEAR WALL PANEL NUMBER PER
STRUCTURAL CALCULATIONS

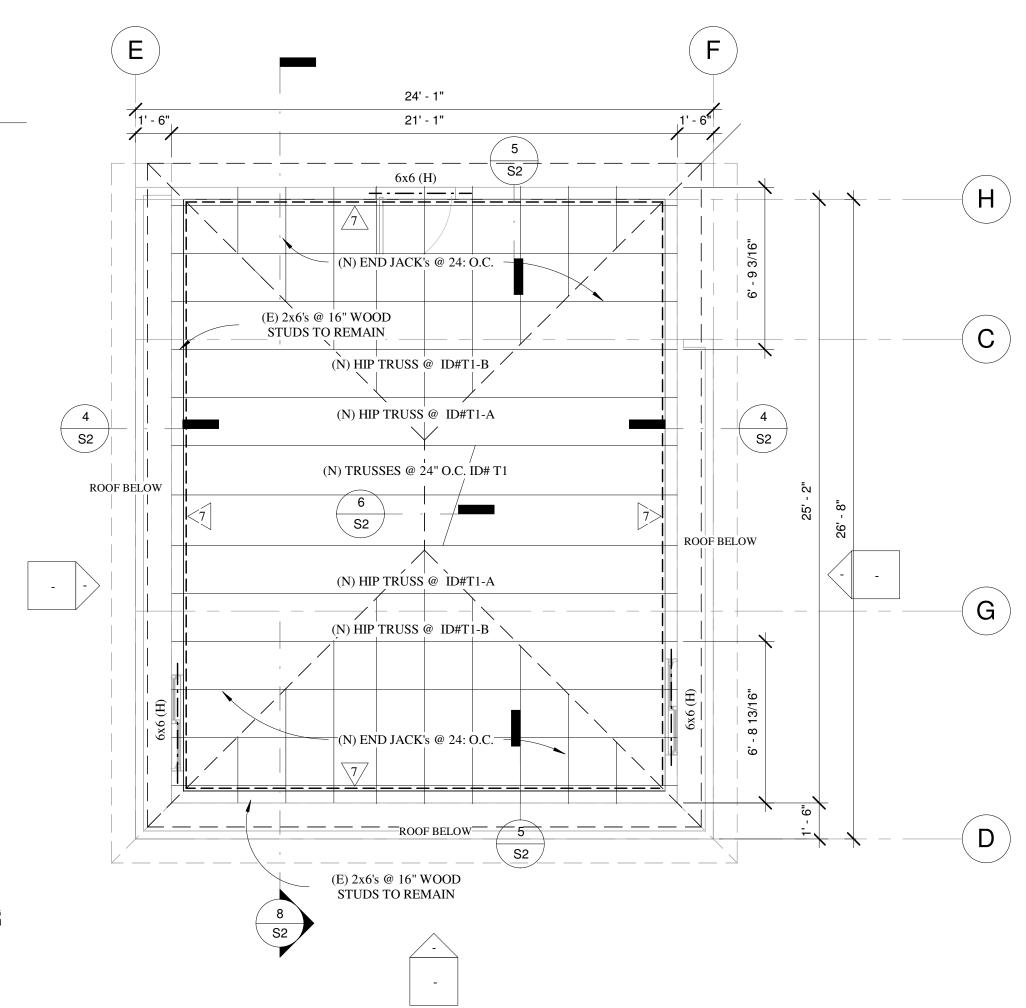
(B). INDICATES BEAM DIRECTLY BELOW JOISTS.
(F). INDICATES BEAM FLUSH WI JOISTS.
(H). INDICATES HEADER.
(L). INDICATES LINTEL.

6. FOR POSTS, POST TO BEAM CONNECTION SEE S1.1

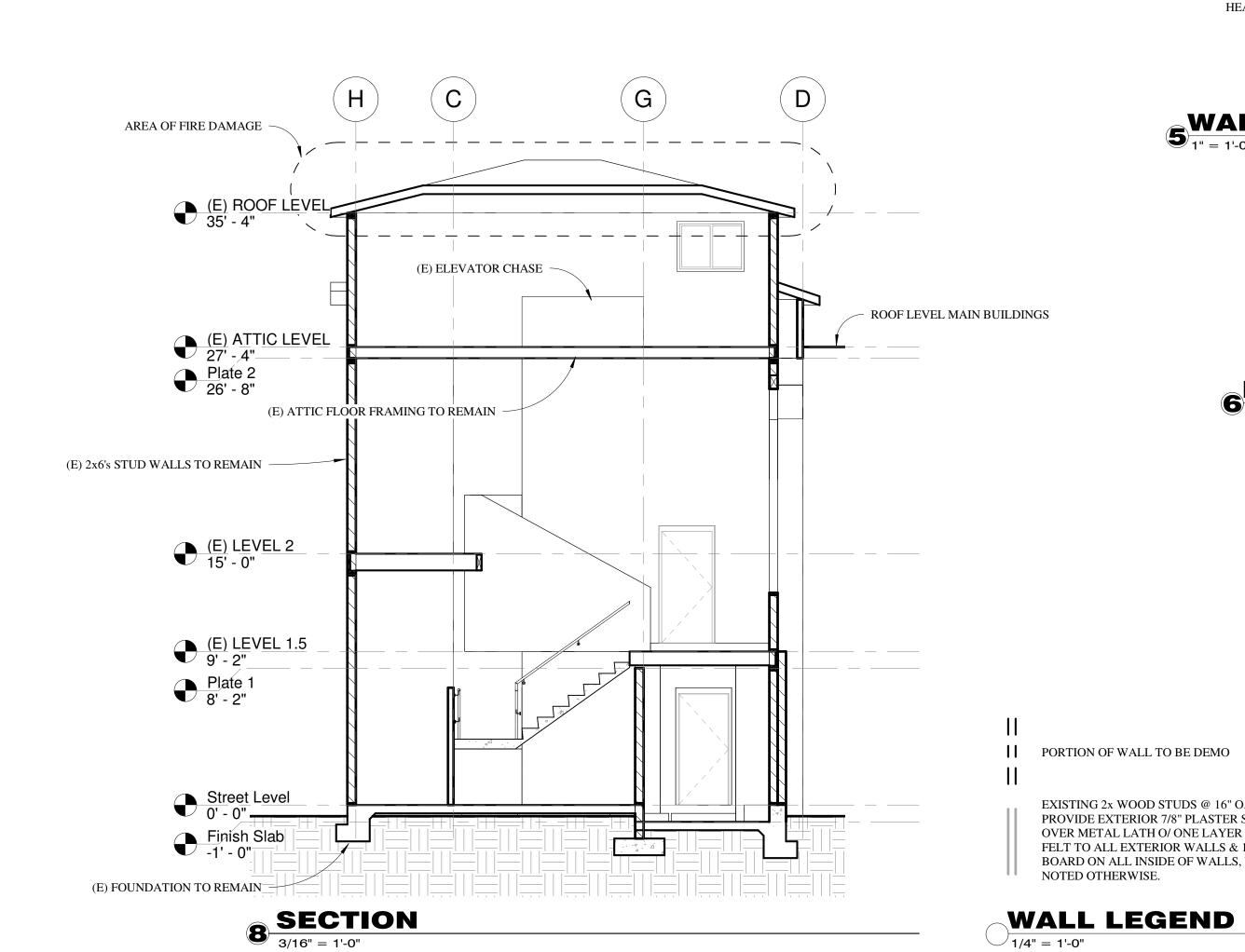
7. DO NOT CUT, NOTCH, DRILL, BORE, SHAVE, TAPER OR FOR ANY REASON MODIFY PRE-ENGINEERED/MANUFACTURED STRUCTURAL ELEMENTS SUCH AS GLUED-LAMINATED MEMBERS, PARALAMS, MICROLAMS, I-JOIST, LIGHT GAUGE METAL MEMBERS AND OTHER SIMILAR TIMBER OR STEEL PRODUCTS OR A LETTER OF CERTIFICATION FROM THE MANUFACTURE'S ENGINEER WITH DETAIL SIGNED AND STAMPED IS ISSUED AND AUTHORIZED BY THE PROJECT ENGINEER OF RECORD AND APPROVED BY THE CITY OF SAN DIEGO BUILDING OFFICIAL.

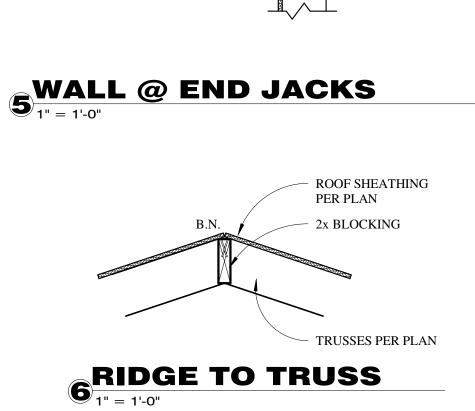
8. DAMAGE TOP PLATE TO BE REPLACED IN KIND PER DETAIL 7 THIS

ROOF FRAM'G NOTES N.T.S.



BOOF FRAMING





PORTION OF WALL TO BE DEMO

EXISTING 2x WOOD STUDS @ 16" O.C.. & PROVIDE EXTERIOR 7/8" PLASTER STUCCO OVER METAL LATH O/ ONE LAYER OF #15 FELT TO ALL EXTERIOR WALLS & 1/2" GYP. BOARD ON ALL INSIDE OF WALLS, UNLESS NOTED OTHERWISE.

TYPICAL ROOF DIAPHRAGM

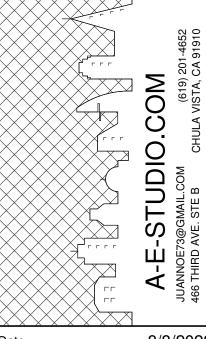
1/2" CDX PLYWD. 1 INDEX (32/16)
10d @ 4" E.N. & B.N. 10d @ 12" F.N.

"DIAPHRAGM SHEATHING NAILS OF OTHER APPROVED SHEATHING CONNECTORS SHALL BE DRIVEN SO THAT THEIR HEAD OR CROWN IS FLUSH WITH THE SURFACE OF THE SHEATHING

IS FLUSH WITH THE SURFACE OF THE SHE

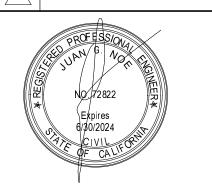
ROOF FRAMING PLAN

1/4" = 1'-0"



ate 8/8/2022

REVISIONS



NORDAN PLAZA 3400 E 8TH ST. NATIONAL CITY, CA 91950

NORDAN PLAZA

Roof Fram'g Plan

S2

EET OF

MiTek USA, Inc. MiTek USA, Inc. 400 Sunrise Avenue, Suite 270 Roseville, CA 95661 Telephone 916-755-3571

Re: nordan

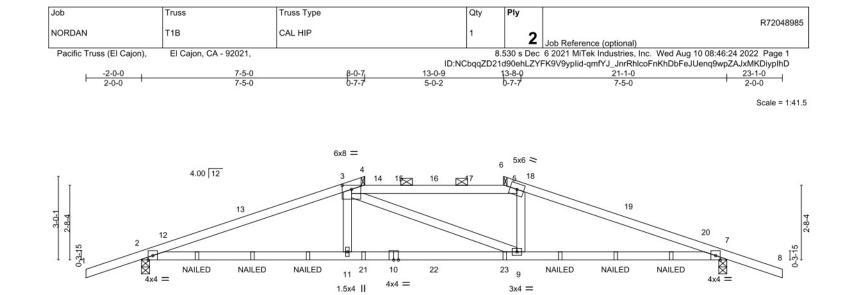
The truss drawing(s) referenced below have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Pacific Truss (El Cajon).

Pages or sheets covered by this seal: R72048983 thru R72048985 My license renewal date for the state of California is June 30, 2024.



August 10,2022 Baxter, David

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



		NAILED		NAILED				
	7-5-0	1	13-8-0			21-1-0		
	7-5-0		6-3-0			7-5-0		1.
Plate Offsets (X,Y)	[3:0-4-0,0-1-12]							
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in (loc)	l/defl L/d		PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.25	TC 0.99	Vert(LL)	-0.13 9-11	>999 240		MT20	220/195
TCDL 15.0	Lumber DOL 1.25	BC 0.76	Vert(CT)	-0.28 9-11	>883 180			
BCLL 0.0 *	Rep Stress Incr NO	WB 0.04	Horz(CT)	0.08 7	n/a n/a			
	Code IBC2021/TPI2014	Matrix-S	, ,			0	Weight: 160 lb	FT = 20%

TOP CHORD

LUMBER-TOP CHORD 2x4 DF No.2 BOT CHORD 2x4 DF No.2 2x4 DF No.2

REACTIONS. (size) 2=0-3-8, 7=0-3-8 Max Horz 2=24(LC 11) Max Uplift 2=-124(LC 12), 7=-124(LC 12)

Max Grav 2=2361(LC 19), 7=2361(LC 19) FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-6179/735, 3-5=-5802/743, 5-7=-6179/734 BOT CHORD 2-11=-637/5824, 9-11=-650/5803, 7-9=-642/5824 3-11=0/488, 5-9=0/488

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows: Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc.

Bottom chords connected as follows: 2x4 - 1 row at 0-9-0 oc Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.

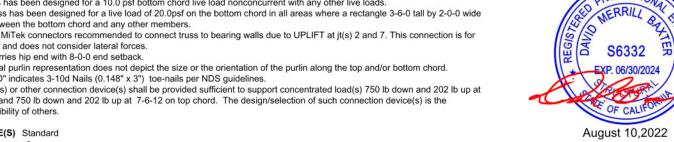
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated. Unbalanced roof live loads have been considered for this design.

4) Wind: ASCE 7-16; Vult=90mph (3-second gust) Vasd=71mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) -2-0-0 to 1-0-0, Interior(1) 1-0-0 to 8-0-7, Exterior(2R) 7-6-12 to 11-9-11 Interior(1) 11-9-11 to 13-6-4, Exterior(2R) 13-0-9 to 17-3-7, Interior(1) 17-3-7 to 23-1-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 5) Provide adequate drainage to prevent water ponding.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Girder carries hip end with 8-0-0 end setback. 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord. 11) "NAILED" indicates 3-10d Nails (0.148" x 3") toe-nails per NDS guidelines.

8) One RT4 MiTek connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 7. This connection is for uplift only and does not consider lateral forces. 12) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 750 lb down and 202 lb up at 13-6-4, and 750 lb down and 202 lb up at 7-6-12 on top chord. The design/selection of such connection device(s) is the responsibility of others.



LOAD CASE(S) Standard ontinued on page 2

Design valid for use only with MTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

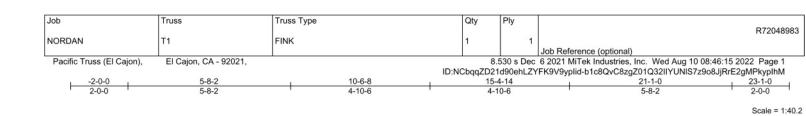
ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information
Safety Information

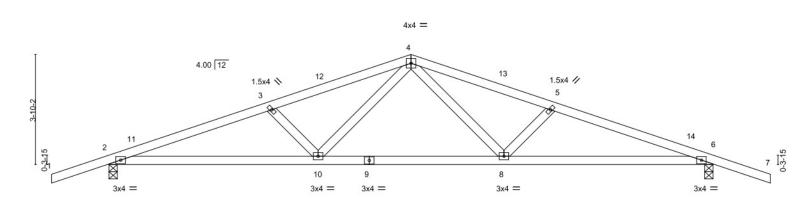


Structural wood sheathing directly applied or 4-10-9 oc purlins,

2-0-0 oc purlins (3-8-2 max.): 3-5.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.





	7-3-9 7-3-9	I	13-9-7 6-5-13	21-1-0 7-3-9	
LOADING (psf) TCLL 20.0 TCDL 15.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code IBC2021/TPI2014	CSI. TC 0.28 BC 0.56 WB 0.10 Matrix-S	DEFL. in (loc) Vert(LL) -0.08 8-10 Vert(CT) -0.20 2-10 Horz(CT) 0.05	>999 240 MT20	GRIP 220/195 FT = 20%

TOP CHORD 2x4 DF No.2 BOT CHORD 2x4 DF No.2 TOP CHORD Structural wood sheathing directly applied or 4-0-15 oc purlins. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. WEBS 2x4 DF No.2

REACTIONS. (size) 2=0-3-8, 6=0-3-8 Max Horz 2=-30(LC 10) Max Grav 2=1086(LC 1), 6=1086(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 2-3=-2141/48, 3-4=-1868/32, 4-5=-1868/32, 5-6=-2141/48 2-10=0/1965, 8-10=0/1339, 6-8=-0/1965 3-10=-380/87, 4-10=0/570, 4-8=0/570, 5-8=-380/87

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Concentrated Loads (lb) Vert: 3=-607 5=-607

Uniform Loads (plf)
Vert: 1-3=-70, 3-4=-70, 3-14=-130, 14-18=-170, 5-18=-130, 5-6=-70, 5-8=-70, 2-7=-49(F=-29)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIL-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=90mph (3-second gust) Vasd=71mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) -2-0-0 to 1-0-0, Interior(1) 1-0-0 to 10-6-8, Exterior(2R) 10-6-8 to 13-6-8 , Interior(1) 13-6-8 to 23-1-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

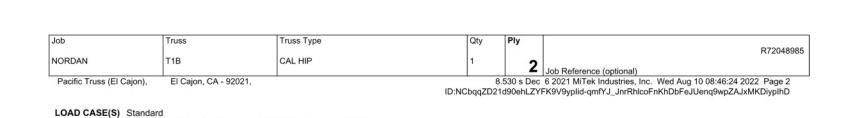


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

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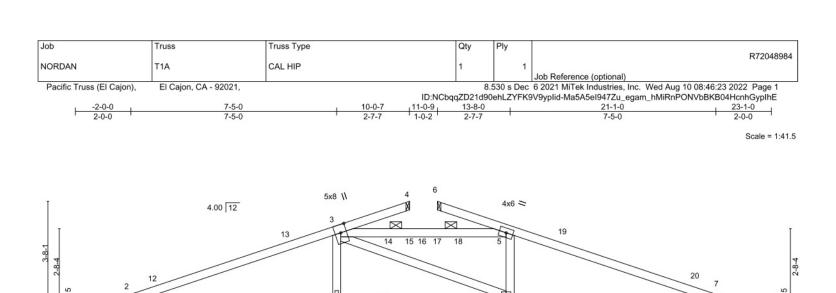
ANS/ITPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information

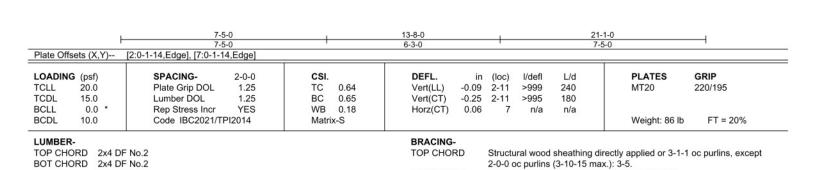
available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





MiTek





3x4 =

BOT CHORD Rigid ceiling directly applied or 9-0-4 oc bracing.

1.5x4 ||

TOP CHORD 2x4 DF No.2 BOT CHORD 2x4 DF No.2 2x4 DF No.2 REACTIONS. (size) 2=0-3-8, 7=0-3-8 Max Horz 2=-29(LC 10)

Max Grav 2=1160(LC 1), 7=1160(LC 1) FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-2377/485, 3-5=-2182/575, 5-7=-2377/485 BOT CHORD 2-11=-466/2191, 9-11=-472/2182, 7-9=-396/2191

Max Uplift 2=-36(LC 12), 7=-36(LC 12)

3-11=0/287, 3-9=-266/266, 5-9=0/287 1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=90mph (3-second gust) Vasd=71mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) -2-0-0 to 1-0-0, Interior(1) 1-0-0 to 10-0-7, Exterior(2R) 7-6-12 to 11-9-11, Interior(1) 11-9-11 to 13-6-4, Exterior(2R) 11-0-9 to 15-3-8, Interior(1) 15-3-8 to 23-1-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate

3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

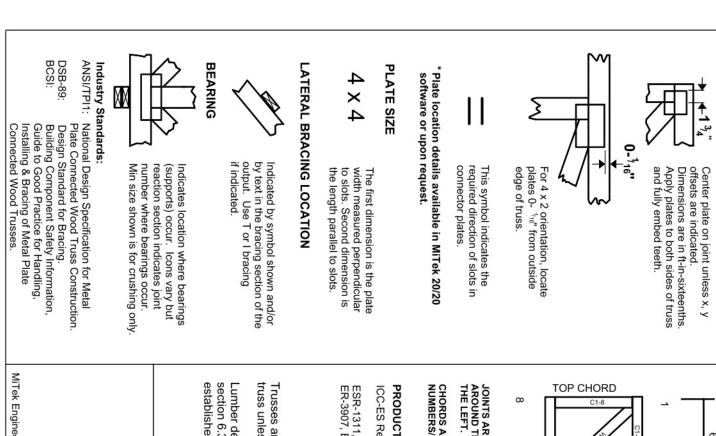
6) One RT4 MiTek connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 7. This connection is for uplift only and does not consider lateral forces. 7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



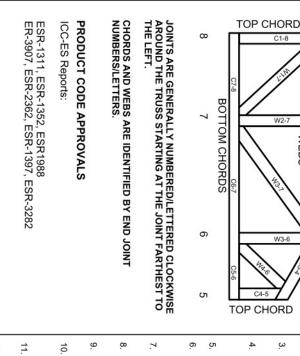
Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

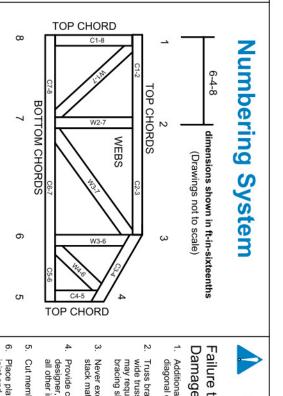
ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information
Safety Information
**available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601











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NORDAN PLAZA

8/8/2022

REVISIONS

Truss Calculations